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Authorised and notified according to Article 29 of the Regulation (EU) No 305/2011 of the European Parliament and of the Council of 9 March 2011

European Technical Assessment ETA-09/0134 of 2014-05-23

I General Part

Technical Assessment Body issuing the ETA and designated according to Article 29 of the Regulation (EU) No 305/2011: ETA-Danmark A/S

Trade name of the construction product:	Gutzeit Angle Brackets (type 89540, 89541, 89550, 89551, 89552, 89553)
Product family to which the above construction product belongs:	Three-dimensional nailing plate (Angle Bracket for timber-to-timber connections)
Manufacturer:	Gutzeit Verbindungssysteme GmbH & Co. Rudolf-Diesel-Strasse 1 D-58730 Fröndenberg, Industriegebiet Tel. +49 2373 - 979261 Fax +49 2373 - 979266 Internet www.gutzeit-holzverbinder.de
Manufacturing plant:	Gutzeit Verbindungssysteme GmbH & Co. Rudolf-Diesel-Strasse 1 D-58730 Fröndenberg, Industriegebiet
This European Technical Assessment contains:	16 pages including 2 annexes which form an integral part of the document
This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of:	Guideline for European Technical Approval (ETAG) No. 015 Three Dimensional Nailing Plates, April 2013, used as European Assessment Document (EAD).
This version replaces:	The previous ETA with the same number issued on 2009-06-03 and expiry on 2014-06-03

II SPECIFIC PART OF THE EUROPEAN TECHNICAL ASSESSMENT

1 Technical description of product and intended use

Technical description of the product
Gutzeit angle brackets with and without rib are one-piece non-welded, face-fixed angle brackets to be used in timber to timber connections. They are connected to the timber elements by a range of profiled nails.

The angle brackets are made from pre-galvanized steel DX 51 D / Z 275 according to EN 10346:2009 with $R_e \geq 295$ N/mm², $R_m \leq 360$ N/mm² and $A_{80} \geq 22\%$ and are available with or without an embossed rib. Dimensions, hole positions and typical installations are shown in Annex A. Gutzeit angle brackets are made from steel with tolerances according to EN 10143.

2 Specification of the intended use in accordance with the applicable EAD

The angle brackets are intended for use in making connections in load bearing timber structures, as a connection between a beam and a purlin, where requirements for mechanical resistance and stability and safety in use in the sense of the Basic Works Requirements 1 and 4 of Regulation (EU) 305/2011 shall be fulfilled.

The connection may be with a single angle bracket or with an angle bracket on each side of the fastened timber member (see Annex A).

The static and kinematical behaviour of the timber members or the supports shall be as described in Annex B.

The wood members may be of solid timber, glued laminated timber and similar glued members, or wood-based structural members with a characteristic density from 290 kg/m^3 to 420 kg/m^3. This requirement to the material of the wood members can be fulfilled by using the following materials:

- Structural solid timber classified to C14-C40 according to EN 338 / EN 14081,
- Glulam classified to GL24-GL36 according to EN 1194 / EN 14080,
- LVL according to EN 14374,
- Parallam PSL,
- Intrallam LSL,
- Duo- and Triobalken,
- Layered wood plates,
- Plywood according to EN 636

Annex B states the load-carrying capacities of the angle bracket connections for a characteristic density of 350 kg/m^3. For timber or wood based material with a lower characteristic density than 350 kg/m^3 the load-carrying capacities shall be reduced by the k_{dens} factor:

$$k_{dens} = \left(\frac{\rho_k}{350} \right)^2$$

Where ρ_k is he characteristic density of the timber in kg/m^3.

The design of the connections shall be in accordance with Eurocode 5 or a similar national Timber Code. The wood members shall have a thickness which is larger than the penetration depth of the nails into the members.

The angle brackets are primarily for use in timber structures subject to the dry, internal conditions defined by service classes 1 and 2 of Eurocode 5 and for connections subject to static or quasi-static loading.

The angle brackets can also be used in outdoor timber structures, service class 3, when a corrosion protection in accordance with Eurocode 5 is applied, or when stainless steel with similar or better characteristic yield and ultimate strength is employed.

The scope of the brackets regarding resistance to corrosion shall be defined according to national provisions that apply at the installation site considering environmental conditions.

The provisions made in this European Technical Assessment are based on an assumed intended working life of the hold downs of 50 years.
The indications given on the working life cannot be interpreted as a guarantee given by the producer or Assessment Body, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

Characteristic	Assessment of characteristic
3.1 Mechanical resistance and stability*) (BWR1)	
Characteristic load-carrying capacity	See Annex B
Stiffness	No performance determined
Ductility in cyclic testing	No performance determined
3.2 Safety in case of fire (BWR2)	
Reaction to fire	The angle brackets are made from steel classified as Euroclass A1 in accordance with EN 1350-1 and EC decision 96/603/EC, amended by EC Decision 2000/605/EC
3.3 Hygiene, health and the environment (BWR3)	
Influence on air quality	No dangerous materials**)
3.7 Sustainable use of natural resources (BWR7)	No Performance Determined
3.8 General aspects related to the performance of the product	The angle brackets have been assessed as having satisfactory durability and serviceability when used in timber structures using the timber species described in Eurocode 5 and subject to the conditions defined by service class 1 and 2
Identification	See Annex A

*) See additional information in section 3.8 – 3.9.

**) In addition to the specific clauses relating to dangerous substances contained in this European technical Assessment, there may be other requirements applicable to the products falling within its scope (e.g. transposed European legislation and national laws, regulations and administrative provisions). In order to meet the provisions of the Construction Products Regulation, these requirements need also to be complied with, when and where they apply.

3.9 Methods of verification

Safety principles and partial factors

The characteristic load-carrying capacities are based on the characteristic values of the nail connections and the joist hangers. To obtain design values the capacities have to be divided by different partial factors for the material properties, the nail connection in addition multiplied with the coefficient k_{mod}.

According to EN 1990 (Eurocode – Basis of design) paragraph 6.3.5 the design value of load-carrying capacity may be determined by reducing the characteristic values of the load-carrying capacity with different partial factors.

Thus, the characteristic values of the load–carrying capacity are determined also for timber failure $F_{Rk,H}$ (obtaining the embedment strength of nails subjected to shear or the withdrawal capacity of the most loaded nail, respectively) as well as for steel plate failure $F_{Rk,S}$. The design value of the load–carrying capacity is the smaller value of both load–carrying capacities.

$$F_{Rd} = \min\left\{\frac{k_{mod} \cdot F_{Rk,H}}{\gamma_{M,H}}; \frac{F_{Rk,S}}{\gamma_{M,S}}\right\}$$

Therefore, for timber failure the load duration class and the service class are included. The different partial factors γ_M for steel or timber, respectively, are also correctly taken into account.

3.10 Mechanical resistance and stability

See annex B for characteristic load-carrying capacities of the joist hangers.

See annex B for the characteristic load-carrying capacity in the different directions F_1 to F_5.

The characteristic capacities of the angle brackets are determined by calculation assisted by testing as described in the EOTA Guideline 015 clause 5.1.2. They should be used for designs in accordance with Eurocode 5 or a similar national Timber Code.

Threaded nails (ringed shank nails) in accordance to EN 14592

In the formulas in Annex B the capacities for threaded nails calculated from the formulas of Eurocode 5 are used assuming a thick steel plate when calculating the lateral nail load-carrying-capacity.

The load bearing capacities of the brackets has been determined based on the use of connector nails 4,0 x 40 mm in accordance with the German national approval for the nails.

The characteristic withdrawal capacity of the nails has to be determined by calculation in accordance with EN 1995-1-1: 2004, paragraph 8.3.2 (head pull-through is not relevant):

$$F_{ax,Rk} = f_{ax,k} \times d \times t_{pen}$$

Where:

$f_{ax,k}$ Characteristic value of the withdrawal parameter in N/mm²

d Nail diameter in mm

t_{pen} Penetration depth of the profiled shank including the nail point in mm, $t_{pen} \geq 31$ mm

Based on tests by Versuchsanstalt für Stahl, Holz und Steine, University of Kalrsruhe, the characteristic value of the withdrawal resistance for the threaded nails used can be calculated as:

$$f_{ax,k} = 50 \times 10^{-6} \times \sigma_k^{\ 2}$$

Where:

σ_k Characteristic density of the timber in kg/m³

The shape of the nail directly under the head shall be in the form of a truncated cone with a diameter under the nail head which exceeds the hole diameter.

The design models allow the use of fasteners described in the table on page 9 in Annex A

No performance has been determined in relation to ductility of a joint under cyclic testing. The contribution to the performance of structures in seismic zones, therefore, has not been assessed.

No performance has been determined in relation to the joint's stiffness properties - to be used for the analysis of the serviceability limit state.

3.11 Aspects related to the performance of the product

3.11.1 Corrosion protection in service class 1 and 2.

The angle brackets are made from pre-galvanized steel DX 51 D / Z 275 according to EN 10346:2009 with $R_e \geq 295$ N/mm², $R_m \leq 360$ N/mm² and $A_{80} \geq 22\%$

3.12 General aspects related to the fitness for use of the product

Gutzeit angle brackets are manufactured in accordance with the provisions of this European Technical Approval using the manufacturing processes as identified in the inspection of the plant by the notified inspection body and laid down in the technical documentation

The nailing pattern used shall be either the maximum or the minimum pattern as defined in Annex A.

The following provisions concerning installation apply:

The structural members – the components 1 and 2 shown in the figure on page 13 - to which the brackets are fixed shall be:

- Restrained against rotation. At a load F_4/F_5, the component 2 is allowed to be restrained against rotation by the Angle brackets.
- Strength class C14 or better, see section II.1 of this ETA
- Free from wane under the bracket.
- The actual end bearing capacity of the timber member to be used in conjunction with the bracket is checked by the designer of the structure to ensure it is not less than the bracket capacity and, if necessary, the bracket capacity reduced accordingly.
- The gap between the timber members does not exceed 3 mm.
- There are no specific requirements relating to preparation of the timber members.

The execution of the connection shall be in accordance with the approval holder's technical literature.

4 Attestation and verification of constancy of performance (AVCP)

4.1 AVCP system

According to the decision 97/638/EC of the European Commission1, as amended, the system(s) of assessment and verification of constancy of performance (see Annex V to Regulation (EU) No 305/2011) is 2+.

5 Technical details necessary for the implementation of the AVCP system, as foreseen in the applicable EAD

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited at ETA-Danmark

Issued in Charlottenlund on 2014-05-23 by

Thomas Bruun
Managing Director, ETA-Danmark

Annex A
Product details definitions

Table A.1 Materials specification

Bracket number	Bracket type	Thickness (mm)	Steel specification	Coating specification
89 540	70x70x55	2,5	DX 51 D	Z 275
89 541	70x70x55 with rib	2,5	DX 51 D	Z 275
89 550	90x90x65	2,5	DX 51 D	Z 275
89 551	90x90x65 with rib	2,5	DX 51 D	Z 275
89 552	105x105x90	3,0	DX 51 D	Z 275
89 553	105x105x90 with rib	3,0	DX 51 D	Z 275

Table A.2 Range of sizes

Bracket number	Bracket type	Height (mm) vertical		Height (mm) horizontal		Width (mm)	
89 540	70x70x55	69	71	69	71	54	56
89 541	70x70x55 with rib	69	71	69	71	51,5	56
89 550	90x90x65	89	91	89	91	64	66
89 551	90x90x65 with rib	89	91	89	91	60	66
89 552	105x105x90	104	106	104	106	89	91
89 553	105x105x90 with rib	104	106	104	106	87	91

Table A.3 Fastener specification

Nail type	**Nail size (mm)**		**Finish**
According to EN 14592	Diameter	Length	
Threaded nail	4,0	40	Electroplated zinc

In the load-carrying-capacities of the nailed connection in Annex B the capacities for threaded nails calculated from the formulas of Eurocode 5 are used assuming a thick steel plate when calculating the lateral nail load-carrying-capacity.
The load-carrying-capacities of the angle brackets have been determined based on the use of connector nails 4,0 x 40 mm in accordance with the German national approval for the nails.
The characteristic withdrawal capacity of the nails has to be determined by calculation in accordance with EN 1995-1-1:2004, paragraph 8.3.2 (head pull-through is not relevant):

$F_{ax,Rk} = f_{ax,k} \times d \times t_{pen}$

Where:
$f_{ax,k}$ Characteristic value of the withdrawal parameter in N/mm^2
d Nail diameter in mm
t_{pen} Penetration depth of the profiled shank including the nail point in mm, $t_{pen} \geq 31$ mm

Based on tests by Versuchsanstalt für Stahl, Holz und Steine, University of Kalrsruhe, the characteristic value of the withdrawal resistance for the threaded nails used can be calculated as:

$f_{ax,k} = 50 \times 10^{-6} \times \rho_k^2$

Where:
ρ_k Characteristic density of the timber in kg/m^3

The shape of the nail directly under the head shall be in the form of a truncated cone with a diameter under the nail head which exceeds the hole diameter.

Annex B
Characteristic load-carrying capacities

Table 1: Force F_1 Column, 2 angle brackets / connection

Bracket number	Bracket type	Nail number n_V	Nail number n_H	$F_{1,Rk}$ [kN] (column)	
				Timber	Steel
89 540	70x70x55	1,2,3	12,13,14,15,16,20,21,22	3,16	1,84
89 541	70x70x55 with rib	1,2	8,9,10,11,13,14	2,34	6,75
89 550	90x90x65	1,2	10,11,12,13,17,18,19,23,24	5,00	2,77
89 551	90x90x65 with rib	1,2	12,13,16,17,21,22	5,00	12,0
89 552	105x105x90	1,2,4,5,6,8,10	18,19,20,21,22,23,26,27,28,30, 35,36	7,52	4,55
89 553	105x105x90 with rib	1,2,4,5,6,7	14,15,16,17,20,21,27,28	5,01	20,3

Table 2: Force F_1 Column, 1 angle bracket / connection

Bracket number	Bracket type	Nail number n_V	Nail number n_H	$F_{1,Rk}$ [kN] (column)	
				Timber	Steel
89 540	70x70x55	1,2,3	12,13,14,15,16,20,21,22	1,58	0,92
89 541	70x70x55 with rib	1,2	8,9,10,11,13,14	1,17	3,37
89 550	90x90x65	1,2	10,11,12,13,17,18,19,23,24	2,50	1,38
89 551	90x90x65 with rib	1,2	12,13,16,17,21,22	2,50	5,98
89 552	105x105x90	1,2,4,5,6,8,10	18,19,20,21,22,23,26,27,28,30, 35,36	3,76	2,28
89 553	105x105x90 with rib	1,2,4,5,6,7	14,15,16,17,20,21,27,28	2,51	10,1

Table 3: Force F_1 Purlin, 2 angle brackets / connection

Bracket number	Bracket type	Nail number n_V	Nail number n_H	$F_{1,Rk}$ [kN] (purlin)	
				Timber	Steel
89 540	70x70x55	1,2,3,7,8	12,13,14,15,16,20,21,22	3,15	1,84
89 541	70x70x55 with rib	1,2,4,5	8,9,10,11,13,14	2,34	6,75
89 550	90x90x65	1,2,4,5,6,7	10,11,12,13,17,18,19,23,24	5,00	2,77
89 551	90x90x65 with rib	1,2,4,5,6,7,8,9	12,13,16,17,21,22	5,00	12,0
89 552	105x105x90	1,2,4,5,6,8,10, 11,12,13,14,15	18,19,20,21,22,23,26,27,28,30, 35,36	7,52	4,55
89 553	105x105x90 with rib	1,2,4,5,6,7,8,9, 10,11	14,15,16,17,20,21,27,28	5,01	20,3

Table 4: Force F_1 Purlin, 1 angle bracket / connection

Bracket number	Bracket type	Nail number n_V	Nail number n_H	$F_{1,Rk}$ [kN] (purlin)	
				Timber	Steel
89 540	70x70x55	1,2,3,7,8	12,13,14,15,16,20,21,22	1,58	0,92
89 541	70x70x55 with rib	1,2,4,5	8,9,10,11,13,14	1,17	3,37
89 550	90x90x65	1,2,4,5,6,7	10,11,12,13,17,18,19,23,24	2,50	1,38
89 551	90x90x65 with rib	1,2,4,5,6,7,8,9	12,13,16,17,21,22	2,50	5,98
89 552	105x105x90	1,2,4,5,6,8,10, 11,12,13,14,15	18,19,20,21,22,23,26,27, 28,30,35,36	3,76	2,28
89 553	105x105x90 with rib	1,2,4,5,6,7,8,9, 10,11	14,15,16,17,20,21,27,28	2,51	10,1

Table 5: Forces $F_{2,3}$, 2 angle brackets / connection

Bracket number	Bracket type	Nail number n_V	Nail number n_H	$F_{2,3,Rk}$ [kN]
				Timber
89 540	70x70x55	1,2,3,7,8	12,13,14,15,16,20,21,22	3,61
89 541	70x70x55 with rib	1,2,4,5	8,9,10,11,13,14	5,53
89 550	90x90x65	1,2,4,5,6,7	10,11,12,13,17,18,19,23,24	6,50
89 551	90x90x65 with rib	1,2,4,5,6,7,8,9	12,13,16,17,21,22	7,43
89 552	105x105x90	1,2,4,5,6,8,10,11, 12,13,14,15	18,19,20,21,22,23,26,27,28, 30,35,36	12,4
89 553	105x105x90 with rib	1,2,4,5,6,7,8,9, 10,11	14,15,16,17,20,21,27,28	10,1

Table 6: Forces $F_{2,3}$, 1 angle bracket / connection

Bracket number	Bracket type	Nail number n_V	Nail number n_H	$F_{2,3,Rk}$ [kN]
				Timber
89 540	70x70x55	1,2,3,7,8	12,13,14,15,16,20,21,22	1,80
89 541	70x70x55 with rib	1,2,4,5	8,9,10,11,13,14	2,77
89 550	90x90x65	1,2,4,5,6,7	10,11,12,13,17,18,19,23,24	3,25
89 551	90x90x65 with rib	1,2,4,5,6,7,8,9	12,13,16,17,21,22	3,72
89 552	105x105x90	1,2,4,5,6,8,10,11, 12,13,14,15	18,19,20,21,22,23,26,27,28, 30,35,36	6,22
89 553	105x105x90 with rib	1,2,4,5,6,7,8,9, 10,11	14,15,16,17,20,21,27,28	5,05

Table 7: Basic Forces $F_{4,5}$, 2 angle brackets / connection

Bracket number	Bracket type	Nail number n_V	Nail number n_H	$F_{4,5,Rk}$ [kN]	
				Timber	Steel
89 540	70x70x55	1,2,3,7,8	12,13,14,15,16,20,21,22	5,83	4,24
89 541	70x70x55 with rib	1,2,4,5	8,9,10,11,13,14	6,13	6,10
89 550	90x90x65	1,2,4,5,6,7	10,11,12,13,17,18,19,23,24	9,30	4,32
89 551	90x90x65 with rib	1,2,4,5,6,7,8,9	12,13,16,17,21,22	7,09	8,51
89 552	105x105x90	1,2,4,5,6,8,10,11, 12,13,14,15	18,19,20,21,22,23,26,27,28, 30,35,36	9,30	8,92
89 553	105x105x90 with rib	1,2,4,5,6,7,8,9, 10,11	14,15,16,17,20,21,27,28	9,87	15,5

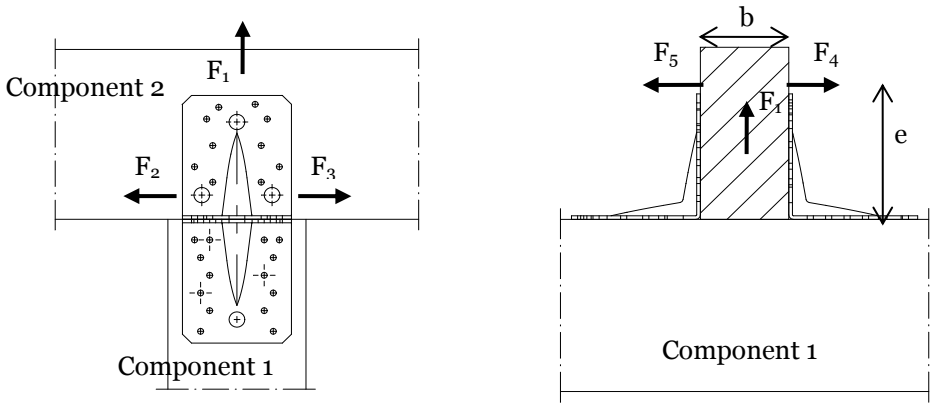
Table 8: Basic Forces F_4, 1 angle bracket / connection

Bracket number	Bracket type	Nail number n_V	Nail number n_H	$F_{4,Rk}$ [kN]	
				Timber	Steel
89 541	70x70x55 with rib	1,2,4,5	8,9,10,11,13,14	6,13	4,52
89 551	90x90x65 with rib	1,2,4,5,6,7,8,9	12,13,16,17,21,22	7,09	6,28
89 553	105x105x90 with rib	1,2,4,5,6,7,8,9,10, 11	14,15,16,17,20,21,27,28	9,87	10,7

Table 9: Basic Forces F_5, 1 angle bracket / connection

Bracket number	Bracket type	Nail number n_V	Nail number n_H	$F_{5,Rk}$ [kN]	
				Timber	Steel
89 541	70x70x55 with rib	1,2,4,5	8,9,10,11,13,14	1,59	1,76
89 551	90x90x65 with rib	1,2,4,5,6,7,8,9	12,13,16,17,21,22	2,30	2,75
89 553	105x105x90 with rib	1,2,4,5,6,7,8,9,10, 11	14,15,16,17,20,21,27,28	2,97	5,42

Definitions of forces, their directions and eccentricity
Forces - Beam to beam connection



Fastener specification
Holes are marked with numbers referring to the nailing pattern in Annex A.

Double angle brackets per connection
The angle brackets must be placed at each side opposite to each other, symmetrically to the component axis.

Acting forces

F_1	Lifting force acting along the central axis of the joint.
F_2 and F_3	Lateral force acting in the joint between the component 2 and component 1 in the component 2 direction
F_4 and F_5	Lateral force acting in the component 1 direction along the central axis of the joint. If the load is applied with an eccentricity e, a design for combined loading is required.

Single angle bracket per connection
Acting forces

F_1	Lifting force acting in the central axis of the angle bracket. The component 2 shall be prevented from rotation. If the component 2 is prevented from rotation the load-carrying capacity will be half of a connection with double angle brackets.
F_2 and F_3	Lateral force acting in the joint between the component 2 and the component 1 in the component 2 direction. The component 2 shall be prevented from rotation. If the component 2 is prevented from rotation the load-carrying capacity will be half of a connection with double angle brackets.
F_4 and F_5	Lateral force acting in the component 1 direction in the height of the top edge of component 2. F_4 is the lateral force towards the angle bracket; F_5 is the lateral force away from the angle bracket. Only the characteristic load-carrying capacities for angle brackets with ribs are given.

Wane
Wane is not allowed, the timber has to be sharp-edged in the area of the angle brackets.

Timber splitting
For the lifting force F_1 it must be checked in accordance with Eurocode 5 or a similar national Timber Code that splitting will not occur.

Combined forces

If the forces F_1 and F_2/F_3 or F_4/F_5 act at the same time, the following inequality shall be fulfilled:

$$\left(\frac{F_{1,d}}{F_{Rd,1}}\right)^2+\left(\frac{F_{2,d}}{F_{Rd,2}}\right)^2+\left(\frac{F_{3,d}}{F_{Rd,3}}\right)^2+\left(\frac{F_{4,d}}{F_{Rd,4}}\right)^2+\left(\frac{F_{5,d}}{F_{Rd,5}}\right)^2 \le 1$$

The forces F_2 and F_3 or F_4 and F_5 are forces with opposite direction. Therefore only one force F_2 or F_3, and F_4 or F_5, respectively, is able to act simultaneously with F_1, while the other shall be set to zero.

If the load F_4/F_5 is applied with an eccentricity e, a design for combined loading **for connections with double angle brackets** is required. Here, an additional force ΔF_1 has to be added to the existing force F_1.

$$\Delta F_{1,d} = F_{4,d} / F_{5,d} \cdot \frac{e}{B}$$

B is the width of component 2.

Gutzeit Angle Brackets

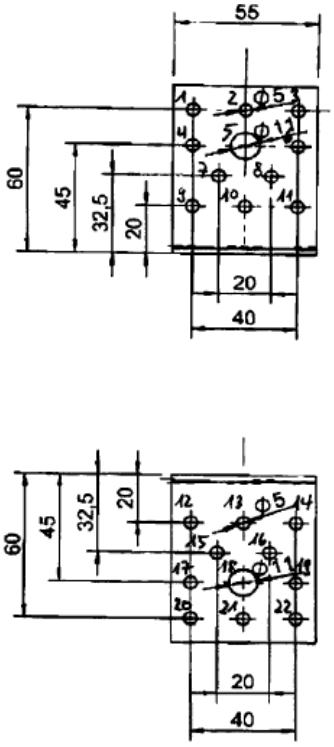


Figure B. 1 Dimensions of Angle Bracket 89 540

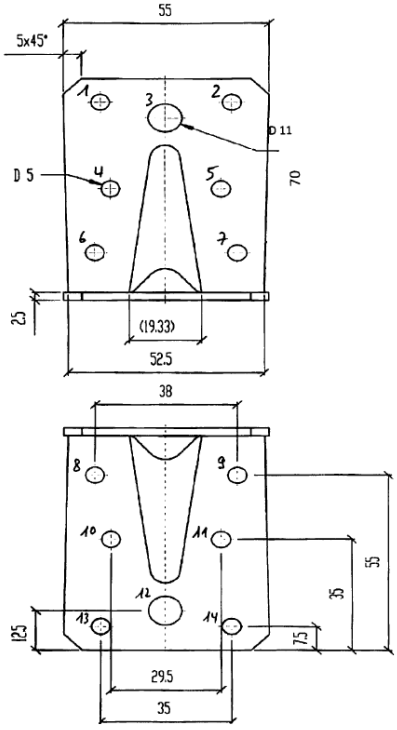


Figure B. 2 Dimensions of Angle Bracket 89 541

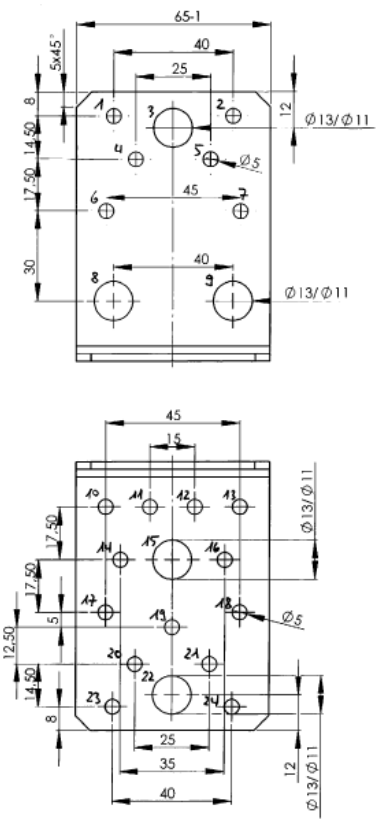


Figure B. 3 Dimensions of Angle Bracket 89 550

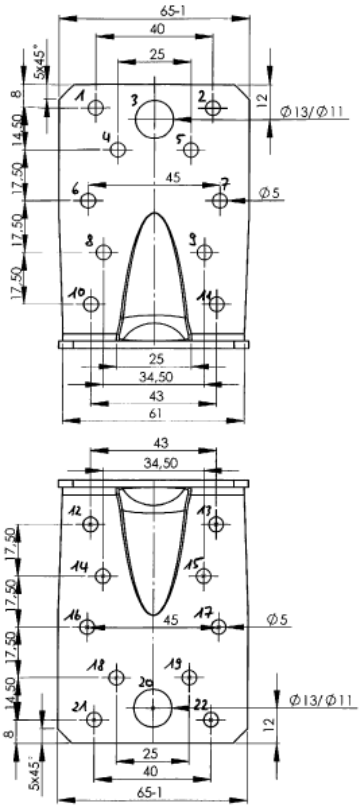


Figure B. 4 Dimensions of Angle Bracket 89 551

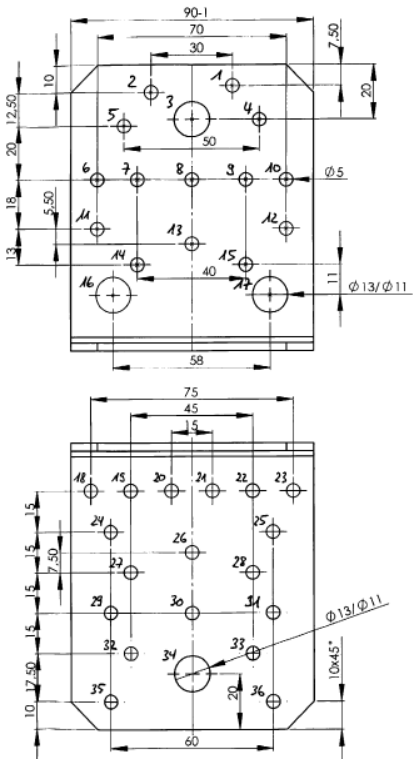


Figure B. 5 Dimensions of Angle Bracket 89 552

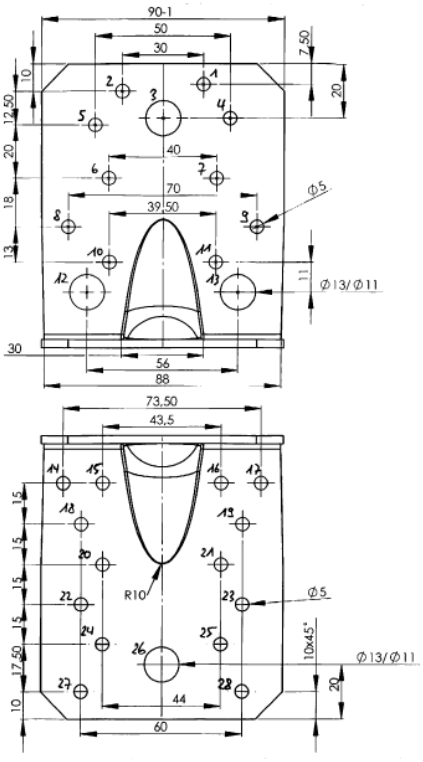


Figure B. 6 Dimensions of Angle Bracket 89 553

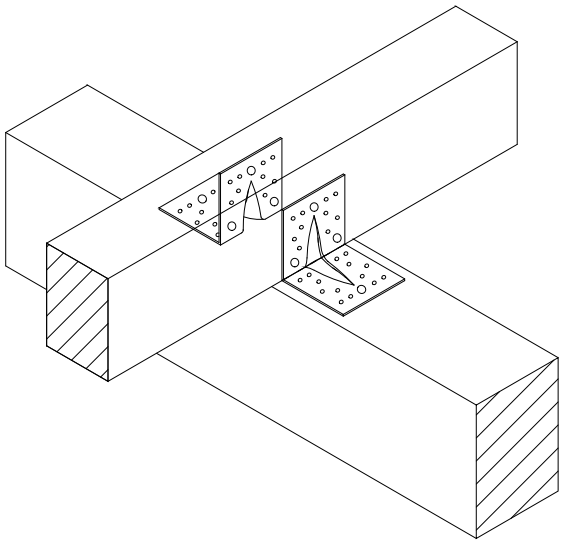


Figure B. 7 Typical installation